

THE ESTIMATION OF DESIGN RAINFALLS FOR SOUTH AFRICA USING A REGIONAL SCALE INVARIANT APPROACH

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Abstract

Accurate estimates of design rainfall are essential inputs in water resources planning and engineering design. The duration of design rainfall which is required for design flood estimation may range from as short as 5 minutes for small urban catchments which have a rapid hydrological response, to a few days for large regional scale flood studies. In South Africa, as in many countries, the spatial density and period of record of rainfall measured manually at daily intervals is far greater than rainfall recorded continuously. However, continuously recorded rainfall is needed to estimate design rainfall for durations shorter than 24 hours. In this study, the scaling characteristics of extreme rainfall and a regional approach, based largely on the more reliable and abundant daily rainfall data, are utilised to estimate design rainfalls in South Africa for durations ranging from 5 minutes to 7 days.

Introduction

The estimation of design flood events are necessary for the planning and design of engineering projects (Rahman *et al.*, 1998). Hence, flood frequency analysis remains a subject of great importance as a result of the economical and environmental impacts of failures of hydraulic structures (Pilgrim and Cordery, 1993; Bobee and Rasmussen, 1995). However, reliable estimates of flood frequency in terms of peak flows and volumes remain a current challenge in hydrology (Cameron *et al.*, 1999). Cordery and Pilgrim (2000) express the opinion that the demands for improved estimates of floods have not been met with any increased understanding of the fundamental hydrological processes.

Standard techniques for flood estimation have been developed for many countries. These generally include statistical analysis of observed peak discharges, where these are available, and event modelling using rainfall-runoff techniques. Observed streamflow data are often not available at the site of interest and rainfall event-based methods frequently have to be used. This requires a probabilistically based estimate of rainfall, generally referred to as design rainfall, to be made at the site of interest. The frequently used term design rainfall is thus the rainfall depth and duration, or intensity, associated with a given probability of exceedance which, in turn, is inversely related to the commonly used term, return period.

Design rainfall depths for various durations are thus required for the many engineering and conservation design decisions made annually in South Africa and which result in many millions of Rands of construction each year. For example, engineers and hydrologists involved in the design of hydraulic structures (e.g. culverts, bridges, dam spillways and reticulation for drainage systems) need to assess the frequency and magnitude of extreme rainfall events in order to generate design flood hydrographs. Hence, Depth-Duration-Frequency (DDF) relationships, which utilise recorded events in order to predict future exceedance probabilities and thus quantify risk and maximise design efficiencies, are a key concept in the design of hydraulic structures (Schulze, 1984).

Numerous regional and national scale studies in South Africa have focused on estimating design rainfalls for durations ≤ 24 h. These include those of Vorster (1945), Woolley (1947), SAWB (1956), Reich (1961), Reich (1963), Bergman and Smith (1973), SAWB (1974), Adamson (1977), Alexander (1978), Midgley and Pitman (1978), Van Heerden (1978), Henderson-Sellers (1980), Schulze (1980), Adamson (1981), Sinske (1982), Op Ten Noort (1983), Schulze (1984), Weddepohl *et al.* (1987), Weddepohl (1988), Smithers (1996), Smithers and Schulze (2000a) and Alexander (2001). Studies in South Africa which have estimated design rainfalls for durations of one day and longer include those by SAWB (1956), Schulze (1980), Adamson (1981), Pegram and Adamson (1988) and Smithers and Schulze (2000b). With the exception of the research by Smithers and Schulze (2000a; 2000b), the other studies have all utilised point design rainfall values using at-site data only and no regionalisation was performed in an attempt to increase the reliability of the design values at gauged sites and for the estimation of design values at ungauged sites.

Smithers and Schulze (2000a) utilised digitised rainfall data from 172 sites in South Africa, each of which had at least 10 years of record, to estimate short duration design rainfall. As illustrated by Smithers (1993) and Smithers and Schulze (2000a), many of the digitised rainfall data are viewed as unreliable as many errors in the digitisation process are evident in the data. In addition, comparisons between the 24 h rainfall totals, computed from the digitised rainfall data, and daily rainfall, as measured at 08:00 every day using standard non-recording raingauges at the same site, indicated numerous significant discrepancies in the two values. Data from 1806 rainfall stations in South Africa which have at least 40 years of quality controlled daily records were utilised by Smithers and Schulze (2000b) to estimate design rainfalls for 1 to 7 day durations in South Africa.

The approach developed by Smithers and Schulze (2002) to overcome the deficiencies in the digitised rainfall data is to utilise, as far as possible, the more abundant and reliable daily rainfall data which generally have longer record lengths than the digitised rainfall data. The objective of this paper is to briefly outline the regional, scale invariant approach developed by Smithers and Schulze (2002) and to illustrate the software developed to estimate design rainfalls at any location in South Africa for durations ranging from 5 minutes to 7 days and for return periods ranging from 2 to 200 years.

Regional Approach to Design Rainfall Estimation

The regional approach to design rainfall estimation developed by Smithers and Schulze (2000a; 2000b) utilises an approach proposed by Hosking and Wallis (1993; 1997), which is based on L-moments. An index storm approach assumes that within a homogeneous region the frequency distribution of observations from all the sites in the region are identical, apart from a site-specific scaling factor. Assuming that N sites form a homogeneous cluster, with site i having a record length n_i , sample mean $l_1^{(i)}$ and sample L-moment ratios $t^{(i)}, t_3^{(i)}, t_4^{(i)}$, then the regional average L-moment ratios t^R, t_3^R, t_4^R , which are weighted in proportion to the sites' record lengths, are computed as:

$$t^R = \sum_{i=1}^N n_i t^{(i)} / \sum_{i=1}^N n_i \quad \dots 1$$

$$t_r^R = \sum_{i=1}^N n_i t_r^{(i)} / \sum_{i=1}^N n_i, \quad r = 3, 4, \dots \quad \dots 2$$

The regional average mean is set to 1 (i.e. $l_1^{(R)} = 1$) and the selected distribution is fitted by equating the theoretical L-moment ratios to $l_1^{(R)}, t^R, t_3^R, t_4^R$ calculated in Equations 1 and 2. As shown in Equation 3, the quantile, with non-exceedance probability F , may be estimated by combining the quantile function of the fitted distribution (\hat{q}), commonly referred to as a growth curve, with the at-site mean.

$$\hat{Q}_i(F) = l_1^{(i)} \hat{q}(F) \quad \dots 3$$

Growth curves thus represent the relationship between design rainfall, scaled by an index value, and event duration. Smithers and Schulze (2002) noted that there are inconsistencies in the 24 h growth curves developed independently using the digitised and daily rainfall databases by Smithers and Schulze (2000a; 2000b) respectively. However, they illustrated that there should be no systematic differences between higher order L-moment ratios for Annual Maximum Series (AMS) extracted using fixed (from daily data) and sliding (from digitised data) windows. It was also established that the 24 h growth curve, derived from digitised data using a sliding 24 h window, and the 1 day growth curve derived from daily rainfall data, should be the same. The differences in the short and long duration growth curves were attributed largely to the non-concurrent periods and different length of records used in the analyses and, to a lesser extent, to the errors in the digitised rainfall data.

Scaling of L-Moment Ratios and Growth Curves

Smithers and Schulze (2000a) developed growth curves for 15 relatively homogeneous regions in South Africa using digitised rainfall data while Smithers and Schulze (2000b) developed growth curves for 78 relatively homogeneous regions in South Africa using daily rainfall data. The regional L-moment ratios computed from the digitised rainfall data for the 15 short duration clusters developed by Smithers and Schulze (2000a) using digitised rainfall data are shown in

Figure 1. Generally, scale invariance of L-CV is a characteristic displayed in most clusters for durations ranging from as short as 10 minutes to 4 days, although there are some exceptions to this trend (Clusters 2, 3, 7 and 8). More deviation from scale invariance was evident and expected for L-skewness. It is noted that the trends in L-CV and L-skewness for durations shorter and longer than 24 h were similar and no distinct shifts in L-CV and L-skewness are evident between 24 and 48 h durations.

The regional average L-moment ratios, computed from the daily rainfall data by Smithers and Schulze (2000b), for selected long duration clusters are shown in Figure 2. From these examples it is evident that scale invariance of both L-CV and L-skewness with duration is a characteristic of daily rainfall for durations ranging from 1 to 7 days in a range of climatic regimes. Deviations from scale invariance for L-CV and L-skewness are postulated for both the digitised and daily rainfall data to be the result of either one, or a combination of, sampling variability, some heterogeneity within a cluster and missing periods of data.

The degree of scale invariance within the digitised and daily rainfall databases is illustrated by computing the slope of the relationship between the regional L-moment ratios and duration for each cluster and for both databases. Durations ranging from 5 minutes to 24 h were used in calculating the slope for the 15 short duration clusters and from 1 to 7 days for the 78 long duration clusters. A frequency analysis of the 15 and 78 slopes for the second (L-CV) and third (L-skewness) regional L-moment ratios was performed and the results are shown in Figure 3. A hypothesis of scale invariance assumes that the slope should be zero.

The mean of the slopes of the relationship between the regional L-moment ratios and duration for durations of 5 minutes to 24 h and for 1 to 7 days computed for the short and long duration clusters respectively, are not significantly different from zero. Hence, it is postulated that the variation in the slopes displayed in Figure 3 are the result of a combination of sampling variability, some heterogeneity within the clusters and periods of missing data.

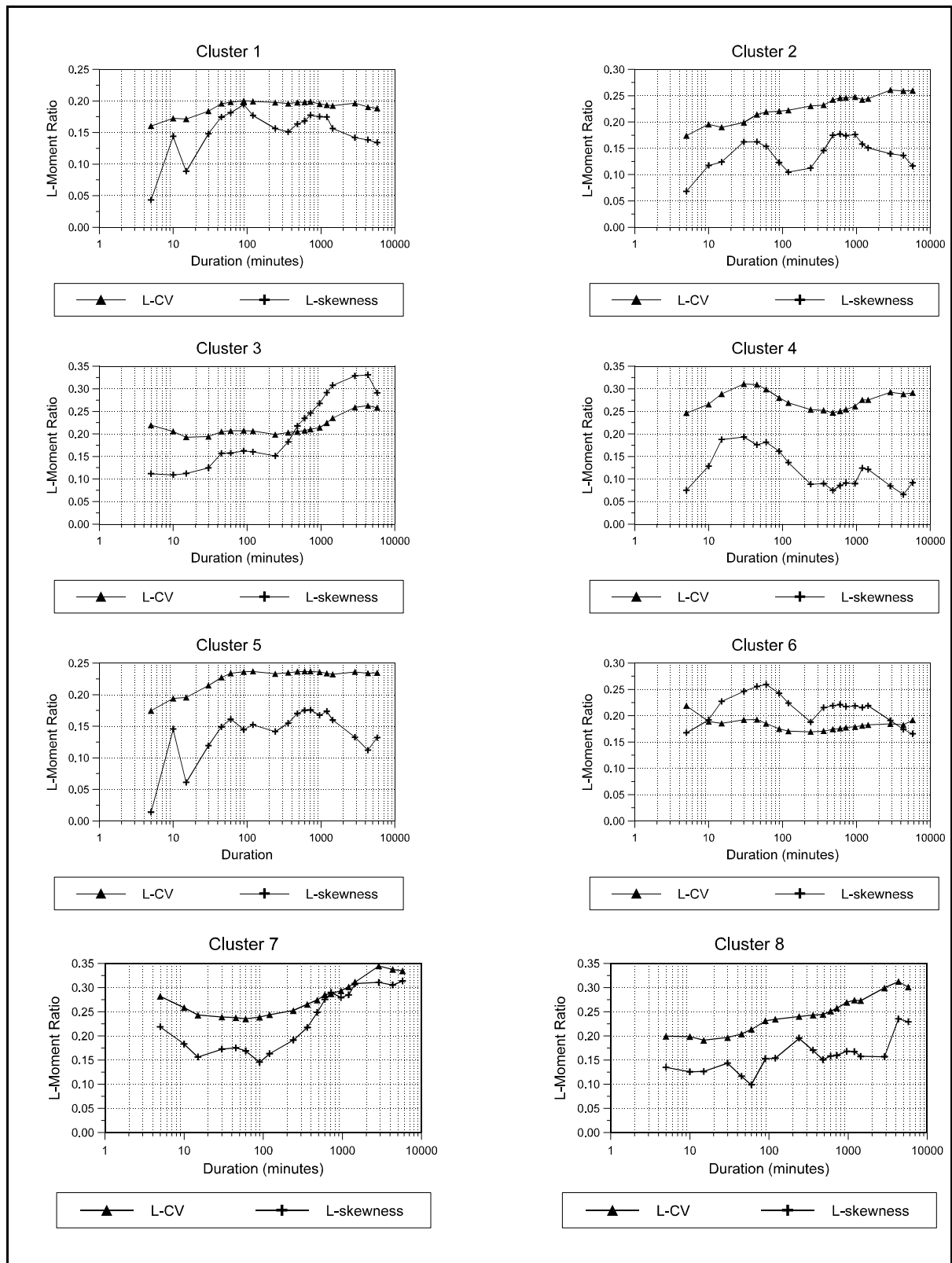


Figure 1 Regional L-moment ratios vs duration for short duration clusters (Smithers and Schulze, 2002)

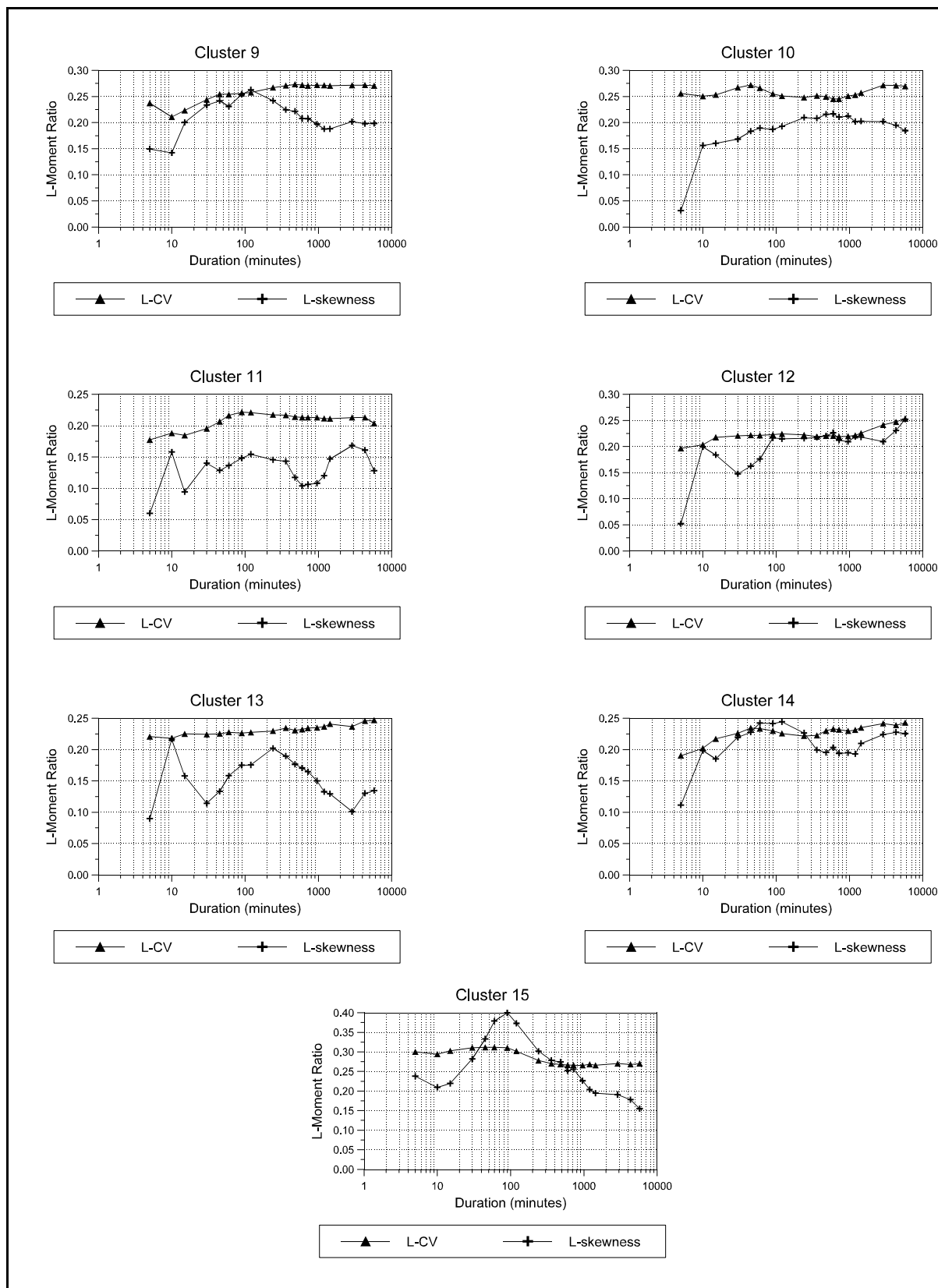


Figure 1 (cont) Regional L-moment ratios vs duration for short duration clusters (Smithers and Schulze, 2002)

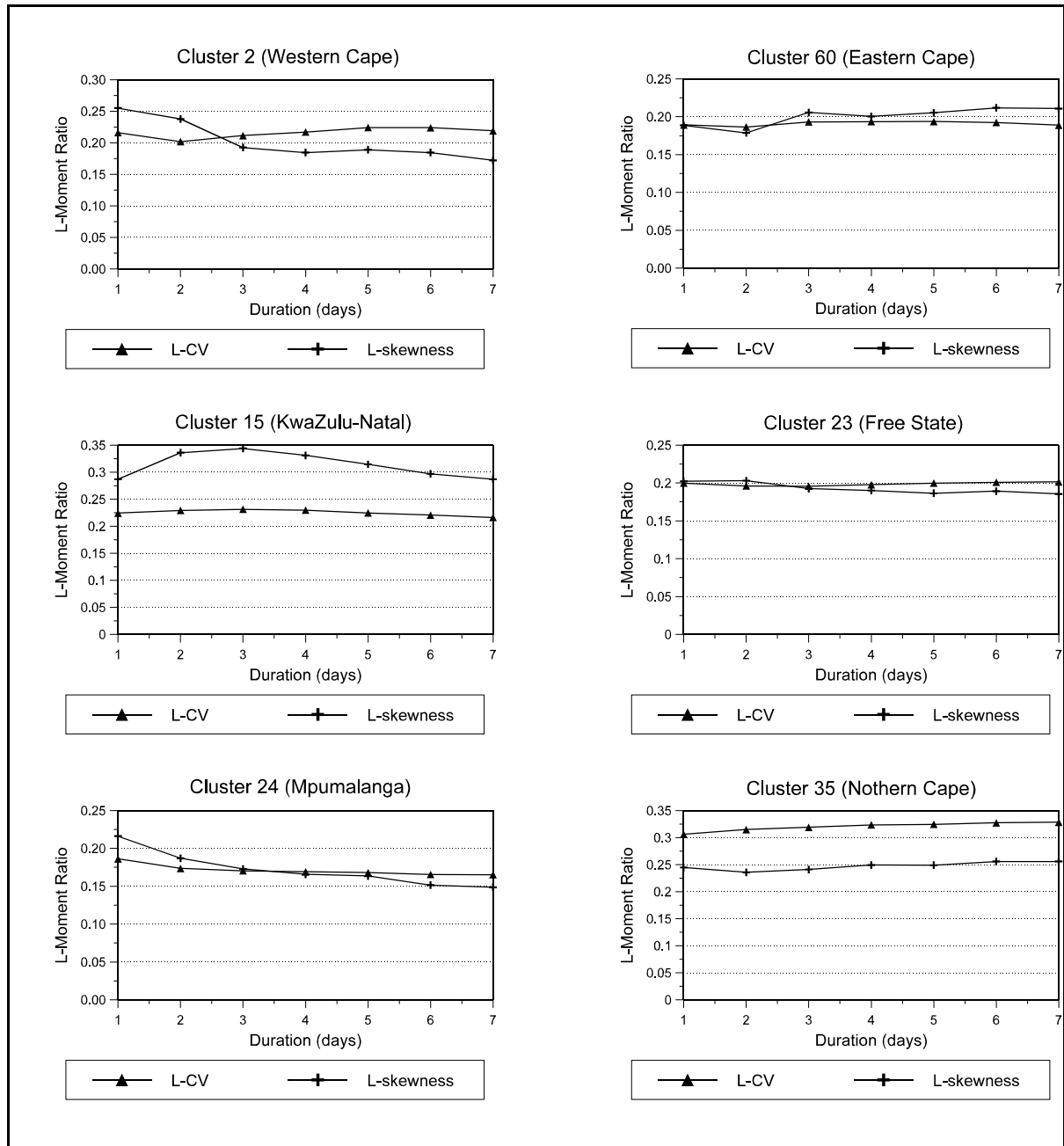


Figure 2 Regional average L-moment ratios vs durations for selected long duration clusters located in different climate regions of South Africa (Smithers and Schulze, 2002)

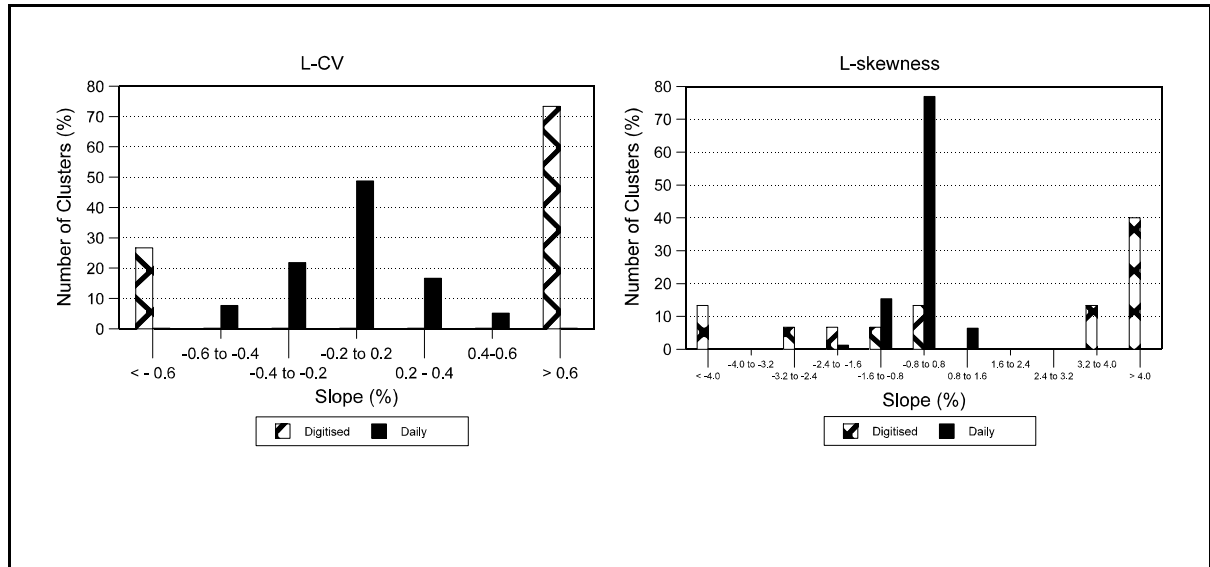


Figure 3 Frequency analysis of the slope of regional average L-moments vs duration computed for short (Digitised) and long duration (Daily) rainfall clusters (Smithers and Schulze, 2002)

Sampling Variability of Extreme Rainfall Data

The sampling variability of the annual maximum rainfall series was estimated by Smithers and Schulze (2002) using three approaches. The first considered sliding windows of data extracted from windows from the entire period of record, the second utilised stochastic modelling of the rainfall process and the third approach implemented a bootstrapping technique. In this paper, only selected results from the bootstrapping technique are presented.

The bootstrapping algorithm described by Adamson *et al.* (1984) and Zucchini and Adamson (1989) was utilised to estimate the sampling variability of the L-moment ratios and hence the errors in the growth curves, based on the assumption of scale invariance. One thousand random samples extracted from the annual maximum series were used to estimate the 90 % confidence intervals. These are illustrated in Figure 4 for Raingauge N23, located in the Ntabamhlope Research Catchments near Estcourt in the KwaZulu-Natal midlands. As expected, and evident in Figure 4, the mean of the 1000 bootstrapped samples (bs) correlates well with the mean of the annual maximum series extracted from the observed data (obs). In addition, at Raingauge N23, both the 24 h L-CV and 24 h L-skewness fall within the 90% confidence interval for durations ranging from 10 minutes to 7 days. Hence, the 24 h growth factors also generally fall within the 90% growth factor confidence intervals for a wide range of durations, as shown for the 2 and 50 year return period rainfalls in Figure 4. Similar results, with a few exceptions, are shown by Smithers and Schulze (2002) in various climatic regions of South Africa.

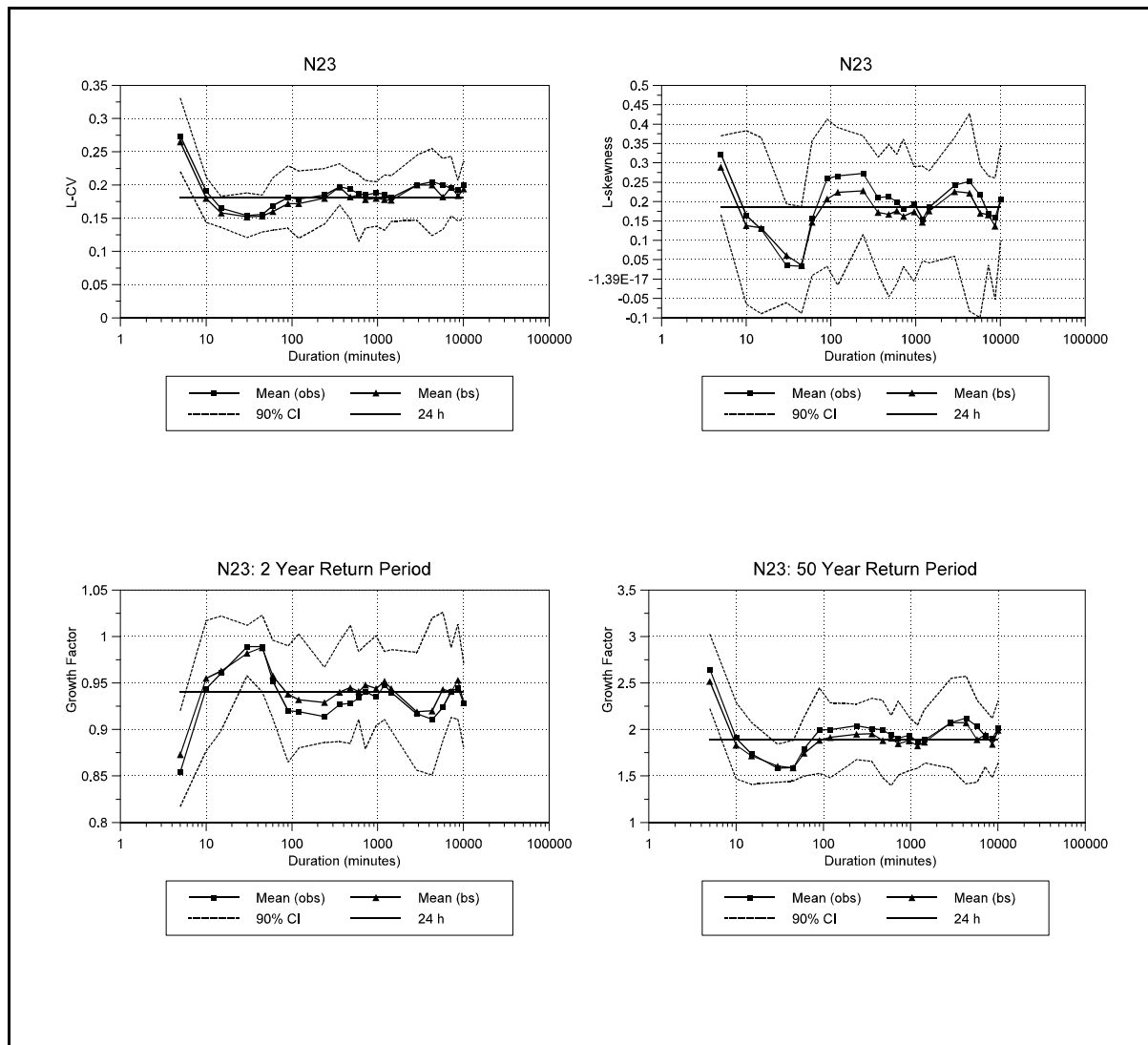


Figure 4 90% Confidence Intervals (CI) for L-moment ratios (top graphs) and 2 and 50 year return period growth factors (bottom graphs) established from 1000 bootstrapped samples at Raingauge N23 (Ntabamhlope)

Based on the detailed evidence provided by Smithers and Schulze (2002) the following are concluded:

- There are no systematic differences between higher order L-moment ratios for annual maximum series extracted using fixed and sliding windows. Hence, the differences in the short and long duration growth curves are attributed largely to the different periods and length of record used in the analyses and to a lesser extent to the errors in the digitised rainfall data.
- It is evident that there is considerable variation with duration in observed higher order L-moments. This is associated with the sampling variability and length and period of record.
- The most reliable estimates of the L-moment ratios are computed from the daily rainfall data which are more abundant and have longer record lengths than the digitised rainfall data.
- For durations ranging from 30 minutes to 7 days, the 1 day L-moment ratios generally fell within the 90% confidence intervals computed for each duration from the observed digitised

rainfall data using a bootstrapping technique. Similar results, not shown in this paper, were obtained by Smithers and Schulze (2002) using both moving window and stochastic rainfall modelling approaches to estimate the sampling variability.

It is thus concluded that the 1 day L-moment ratios, and hence growth curves, are the most reliable estimate of the L-moment ratios for all durations. Design rainfall estimates for all durations may thus be estimated as the product of the 1 day growth curves and an estimate of the mean of the annual maximum series (index value) at the site and for the duration in question. The methodology developed for estimating the mean of the annual maximum series at ungauged locations in South Africa is summarised in the following section.

Estimation of the Mean of the Annual Maximum Series at Ungauged Locations in South Africa

The methodology developed by Smithers and Schulze (2002) for estimating the mean of the annual maximum series for all durations at an ungauged location is dependent on the mean of 1 day annual maximum series. Using a cluster analysis of site characteristics, the 78 long duration clusters were grouped into 7 regions for the estimation of the mean of the 1 day annual maximum series. Multiple linear regression relationships with site characteristics (MAP, latitude, altitude) as independent variables enabled the mean of the 1 day annual maximum series to be estimated at any location in South Africa. Gridded residual errors at stations which had at least 40 years of data were used to correct the estimated values at these sites and ensure that the estimated values were the same as the observed values at these sites. This approach was shown to result in reliable and consistent estimates of the 1 day annual maximum series at sites not used in the analysis.

For durations longer than 1 day, the mean of the D day ($2 \leq D \leq 7$) duration values were noted to scale linearly as a function of the 1 day values. The parameters of the regression were found to scale (by a power law relationship) with duration, and three parameters were derived to describe the relationship between the two regression parameters (coefficient and intercept) and duration. Thus, 6 parameters in all were derived for each of the 7 regions which enable the estimation of the mean of the annual maximum series for durations ranging from 2 to 7 days at any location in South Africa.

For durations shorter than 1 day, the mean of the H minute ($H \leq 1440$) duration values were found to scale linearly as a function of the 24 h values. Thus, for each of the 15 short duration clusters and for 15 durations ranging from 5 to 1200 minutes, linear regression coefficients were derived.

The approach adopted to estimate the mean of the annual maximum series for any duration is a two step process. Firstly, the mean of the 1 day annual maximum series is estimated at the required location using regionalised regressions. Secondly, the mean of the annual maximum series for durations longer than 1 day are scaled directly from the 1 day value. For durations shorter than 1 day, the values are scaled from the 24 h value, which in turn is estimated directly from the 1 day value for the location. This approach for durations < 24 h was shown to be more efficient and reliable than the methodology developed by Smithers and Schulze (2000a). The process is illustrated schematically in Figure 5.

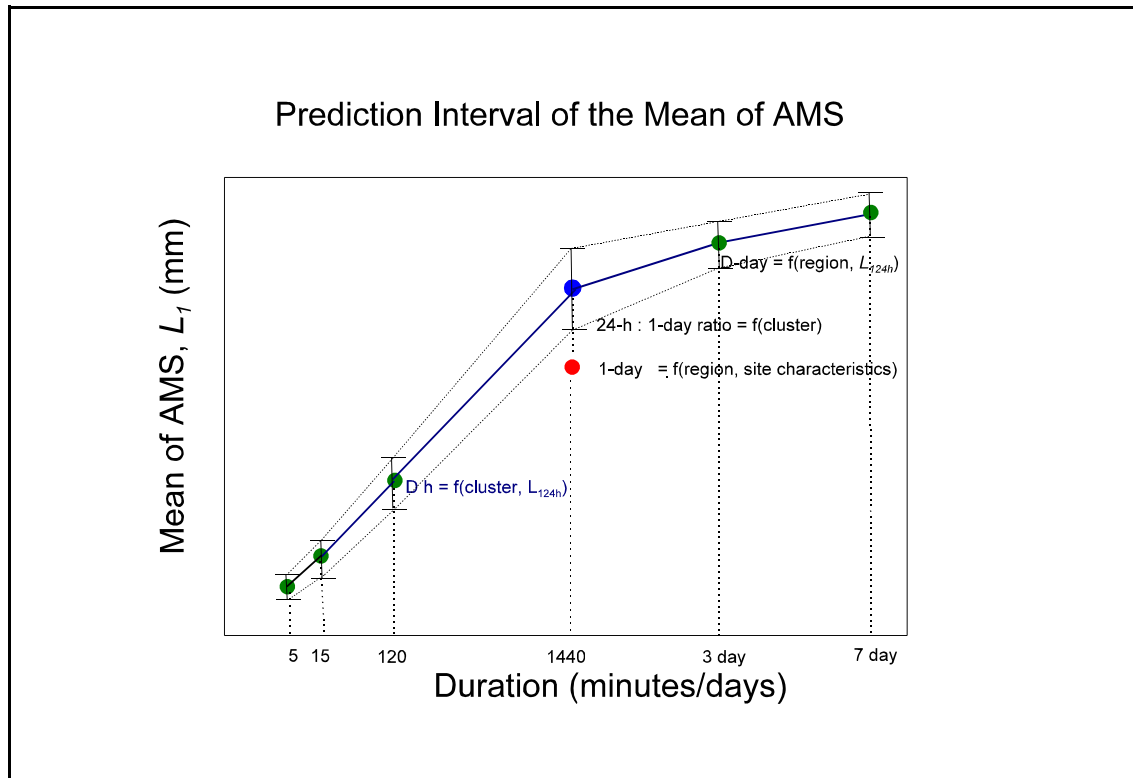


Figure 5 Schematic representation of the estimation of the mean of the annual maximum series at ungauged locations in South Africa

Software Developed

A computer programme with a graphical user interface has been developed in Java to facilitate the estimation of design rainfall depths for any location in South Africa. This software implements the procedures developed by Smithers and Schulze (2002), which are termed the Regional L-Moment Algorithm and Scale Invariance (RLMA&SI), and enables the estimation of design rainfalls at a spatial resolution of 1 arc minute and for durations ranging from 5 minutes to 7 days and for return periods of 2 to 200 years. The graphical user interface developed is shown in Figure 6 and typical output from the software is shown in Figure 7. The software can be downloaded from <http://www.beeh.unp.ac.za/HydroRisk/>.

Discussion and Conclusions

The estimation of reliable and consistent estimates of design rainfall are an essential element in the efficient design of hydraulic structures. Unlike previous studies which used a single site approach, Smithers and Schulze (2000a; 2000b) developed a regional approach to design rainfall estimation in South Africa. However, much of the focus of the work by Smithers (1998) to estimate design rainfalls for durations ≤ 24 h in South Africa was to compensate for the relatively unreliable digitised rainfall data recorded at relatively few stations and which have short record lengths. Hence the RLMA&SI procedures focus on using the more abundant and reliable daily

rainfall data.

The RLMA&SI procedures developed by Smithers and Schulze (2002) for design rainfall estimation in South Africa, and summarised in this paper, utilises:

- a regional approach,
- growth curves derived from the daily rainfall database, which has far more stations with longer record lengths than those of the digitised rainfall database,
- the scaling characteristics of extreme rainfall, and
- a regional method which is robust and reliable to estimate the mean of annual maximum series and which is scaled from values estimated from the reliable and abundant daily rainfall data.

In conclusion, the development of the RLMA&SI procedures by Smithers and Schulze (2002) for design rainfall estimation in South Africa not only adopts a novel approach by utilising the scale invariance of growth curves with duration, but enables reliable and consistent estimates of design rainfall to be made in South Africa by means of a Java-based computer programme with a graphical user interface.

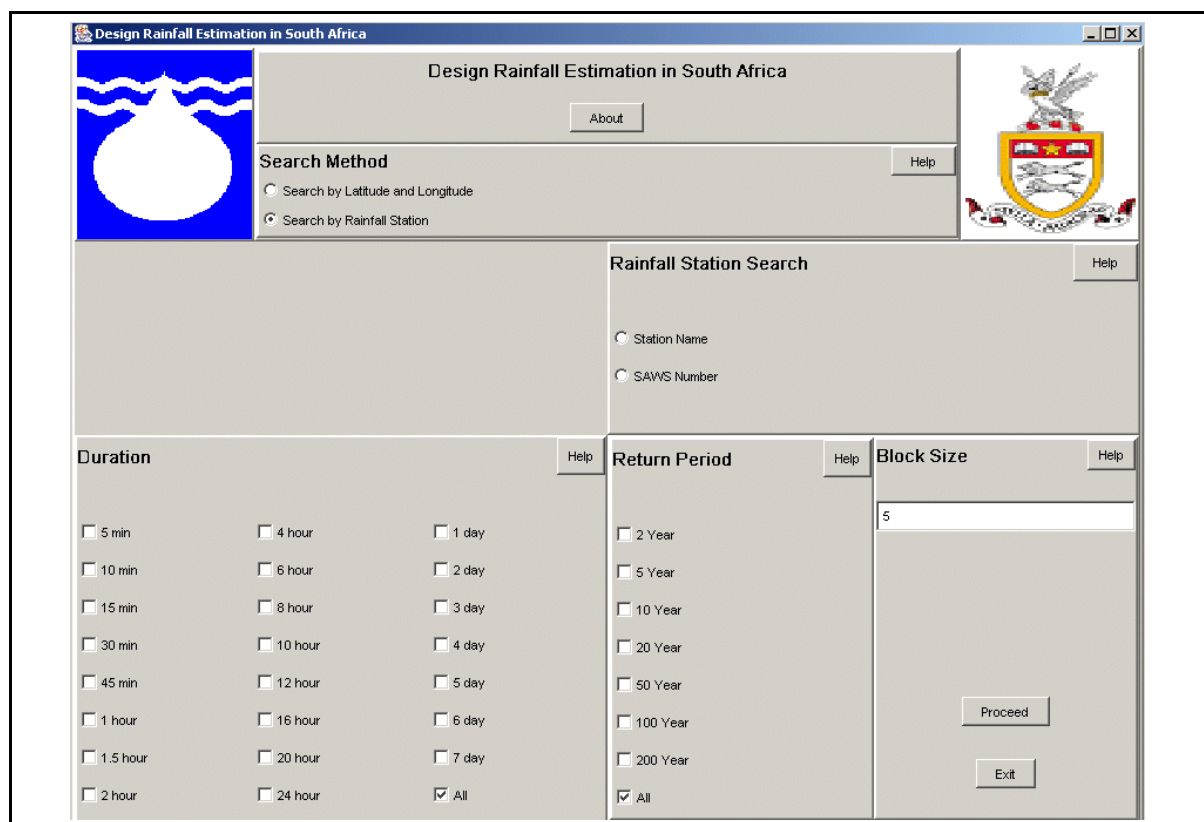


Figure 6 Graphical user interface for the estimation of design rainfall in South Africa

User selection has the following criteria:
Station Name: cedara
Durations requested: 5 m, 10 m, 15 m, 30 m, 45 m, 1 h, 1.5 h, 2 h, 4 h, 6 h, 8 h, 10 h, 12 h, 16 h, 20 h, 24 h, 1 d, 2 d, 3 d, 4 d, 5 d, 6 d, 7 d
Return Periods requested: 2 yr, 5 yr, 10 yr
Block Size requested: 0 minutes

Data extracted from Daily Rainfall Estimate Database File
The station selected and the five closest stations are listed

Station Name	SAWB Number	Distance (km)	Record (Years)	Latitude (°)	Longitude (°)	MAP (mm)	Altitude (m)	Duration (m/h/d)	Return Period (years)									
									2	2L	2U	5	5L	5U	10	10L	10U	
CEDARA COLLEGE	0239482_W	0.0	40	29	32	30	17	876	1 d	55.6	55.2	55.8	77.8	77.3	78.2	95.2	94.0	96.1
									2 d	70.0	69.4	70.6	98.5	97.6	98.9	122.1	120.5	123.7
									3 d	79.8	79.1	80.6	112.7	111.7	113.1	140.2	138.2	142.2
									4 d	87.4	86.5	88.2	123.1	122.0	123.7	152.6	150.0	154.9
									5 d	92.0	91.1	92.8	128.8	127.6	129.4	158.6	155.7	160.9
									6 d	99.0	98.1	100.0	138.1	136.7	139.0	168.9	165.8	171.3
									7 d	102.5	101.6	103.5	142.0	140.7	143.0	172.8	169.7	175.3
.																		
BOTANIC GARDENS - PMB	0239605_P	9.0	83	29	35	30	21	1001	1 d	57.0	56.1	57.5	83.0	82.0	83.5	104.7	102.8	106.7
									2 d	73.0	71.7	74.3	106.0	104.6	106.8	133.7	130.3	136.8
									3 d	84.0	82.2	85.7	122.3	120.5	123.4	154.4	150.0	158.0
									4 d	91.5	89.7	93.4	132.0	130.1	133.2	165.5	160.9	170.1
									5 d	98.4	96.4	100.3	140.2	138.3	141.7	174.4	169.6	178.6
									6 d	104.8	103.0	106.6	148.1	146.0	149.5	182.8	178.1	187.1
									7 d	110.7	109.0	112.6	154.9	152.9	156.4	190.3	185.6	194.6
.																		
Gridded values of all points within the specified block																		
Latitude (°)	Longitude (°)	MAP (mm)	Altitude (m)	Duration (m/h/d)	Return Period (years)													
29	32	30	17	876	1134	5 m	11.5	7.4	15.6	16.1	10.3	21.9	19.7	12.5	26.9			
						10 m	15.5	10.6	20.4	21.7	14.9	28.6	26.6	18.1	35.1			
						15 m	18.5	13.5	23.5	25.9	18.8	32.9	31.7	22.9	40.5			
						30 m	23.3	17.2	29.4	32.6	24.1	41.2	39.9	29.3	50.6			
						45 m	26.7	20.2	33.2	37.3	28.2	46.4	45.7	34.4	57.1			
						1 h	29.3	22.6	36.1	41.1	31.6	50.6	50.3	38.4	62.2			
						1.5 h	33.6	26.1	41.1	47.0	36.5	57.6	57.6	44.4	70.8			
						2 h	37.0	29.1	44.9	51.8	40.7	62.9	63.3	49.5	77.3			
						4 h	42.9	34.8	51.0	60.1	48.8	71.5	73.5	59.3	87.9			
						6 h	46.8	40.0	53.6	65.6	56.0	75.1	80.2	68.2	92.4			
						8 h	49.8	44.0	55.7	69.8	61.5	78.0	85.4	74.9	95.9			
						10 h	52.3	47.5	57.0	73.2	66.4	79.9	89.6	80.9	98.2			
						12 h	54.4	50.3	58.3	76.2	70.5	81.7	93.2	85.7	100.5			
						16 h	57.1	53.5	60.5	79.9	74.9	84.8	97.7	91.1	104.2			
						20 h	60.7	57.5	63.8	85.0	80.4	89.4	104.0	97.9	109.9			
						24 h	63.1	59.8	66.4	88.4	83.6	93.0	108.2	101.8	114.3			
						1 d	53.6	43.0	64.1	75.0	60.2	89.7	91.7	73.3	110.3			
						2 d	66.1	61.2	70.8	92.5	85.7	99.2	113.2	104.3	121.9			
						3 d	74.7	67.7	81.6	104.6	94.7	114.3	127.9	115.2	140.6			
						4 d	81.2	72.7	89.6	113.7	101.7	125.5	139.1	123.8	154.4			
						5 d	86.6	76.6	96.6	121.3	107.2	135.3	148.4	130.4	166.3			
						6 d	91.3	79.6	102.9	127.9	111.5	144.2	156.5	135.7	177.3			
						7 d	95.5	82.3	108.7	133.7	115.1	152.3	163.6	140.1	187.2			
...																		

Figure 6 Example of design rainfall output

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